

Stress Analysis of Buried Pipeline Installed by Horizontal Directional Drilling Using ANSYS Finite Element Software

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Abstract

In designing a horizontal directional drilling (HDD) pipeline project, designers face the challenge of determining the regions of maximum and minimum stresses on pipelines, ensuring the stability of the bore-hole from collapse and minimizing the stresses induced on the pipeline due to the bore-profile. This study analyses the stress induced on an HDD pipeline system using the ANSYS Version 18, mechanical APDL finite element (FE) software. The pipeline used as the case study was a gas transmission pipeline installed in south-west Nigeria. A macro-file for ANSYS Version 18, mechanical APDL used to model the pipeline was developed. The results showed that the maximum and minimum stresses induced on the HDD pipeline were at the top and bottom of the pipe, respectively; while the stresses on the sides were uniform (≈888 kg/cm²) all through the pipeline, irrespective of element number. The maximum stress occurred at the curvature point with the highest entry angle (10°), resulting in a maximum deflection at this point. The model stress validation performed by comparing results with theoretical solutions, both with respect to radius of curvature and internal pressure, showed percentage difference (errors) less than 10%. The cross sectional area validation showed a percentage difference of 0.059%.

Keywords

Horizontal Directional Drilling (HDD), Finite Element Analysis (FEM), Finite Element Analysis (FEA), ANSYS Mechanical APDL

1. Background

1.1. Stresses on Buried Pipeline

The loads acting on the pipe include its weight, the internal operating pressure

and the external pressure due to the overhead soil load [1]. Underground loads on a buried pipeline induced by the soil pressure can be divided into two parts. Firstly, there is the soil pressure which increases with the depth of cover. Then, there is the additional force caused by ground loads which decreases with increase in depth of cover [2]. The depth of embedment of pipe, type of backfill, the pipe thickness and surcharge loads are the prime factors that affect the behavior of buried pipes [3].

HDD pipelines are subjected to a combination of tension, bending, and external pressure. Drilled path design and pipe specification may be governed by these installation loads, as they could be more severe than operational loads [4]. The borehole profile and pipe properties must be considered in selecting the suitable pipe materials for an HDD installation. These two factors should be considered together to ensure that the pipeline can be installed and operated without risk of damage [5].

1.2. Finite Element Method (FEM) for Pipeline Stress Analysis

In the analysis and design of underground structures, the finite element method (FEM) has been used extensively as an effective numerical technique. The results more reasonably reflect the actual situation, providing an improved design procedure to avoid pipe wear and breakage [1].

Its application ranges from the study of aircraft or automobile structural framework, complicated thermal system (nuclear power plant), to the analysis of a fluid flowing through a duct, over a weir, or through the earth [6] [7] [8] developed a 3D FE program using MATLAB to examine the performance of buried pipeline subjected to large ground motion; crossing active fault.

Spectrum technique with 1D beam units was used by Wu *et al.* [9] to undertake dynamic earthquake analysis on oil pipelines using CAESAR II stress analysis software. Hence, critical sections and maximum stress region were identified visibly. It was concluded that reducing the bend angle could minimize the stress and displacement of underground pipelines during an earthquake. However, this study was not extended to stress analysis of deeply buried HDD pipelines. Also, Huang, *et al.* [10] utilized CAESAR II software to investigate the stress on underground oil pipelines in steep and high slope regions. Comparing the different simulation results showed that the movement and stress of underground oil pipelines in steep and high slope regions were drastically reduced with enhanced safety by proposing an improved laying process.

Gabar and Bilgin [11] used ABAQUS FE software package to model buried pipeline behavior under surface-induced actions, and described the pipe-soil interactions and pipe deformation. They concluded that vertical pipe deformation increased with increasing loads for various soil layers, which may cause buckling.

Xiao, *et al.* [1] performed analysis of glass reinforced plastics (GRP) pipeline using ANSYS and the results were generally consistent with that of the traditional CECS190 standard, based on the maximum long term vertical deformation. It

was further concluded that FEM may be utilized for more complex underground pipes structures.

Nirmala and Rajkumar [3] also used ANSYS as FEA tool on buried UPVC pipe and suggested that the depth of backfill increases the deflection of buried pipes, and that the behaviour of buried pipes varied due to the type of backfill material. Hence, it can be observed that ANSYS has not been used as an FEA tool for HDD pipeline stress analysis.

2. Materials and Methods

The model was created using ANSYS, Version 18, Mechanical APDL following the flow diagram in **Figure 1**. The co-ordinates for the key points, exit angle, entry angle, element type and the geometric properties (such as, pipe outer diameter, thickness, length of crossing etc.); and the material properties of the steel pipe (such as, the specified minimum yield stress, poisson ratio and young modulus) were all inputted into the model at the pre-processing stage.

At the solution stage, the loads and boundary conditions such as the degree of freedom and model solution setting were clearly specified. The final stage of the model, post-processing stage, was where the deflection plots and the stress contour diagrams of the pipeline system were plotted.

2.1. Design Input Parameters

The pipeline used to create the model on ANSYS, Version 18, Mechanical APDL, was a gas pipeline installed by Horizontal Directional Drilling (HDD) in



Figure 1. ANSYS Mechanical APDL design procedure flowchart.

south-west Nigeria. The gas pipeline design parameters are shown in Table 1.

These parameters were input into the ANSYS platform to give the section outline in **Figure 2**. The ANSYS, Version 18, Mechanical APDL solver was used to model the pipeline. A macro file for non-linear analysis was prepared for modeling the pipeline system.

2.2. Modeling and Meshing

Four (4) co-ordinates points were identified on the pipeline design profile to

Table 1. Design parameters for the gas pipeline.

Parameter	Value [units]
Pipe Diameter, OD	91.44 [cm]
Wall Thickness, <i>t</i>	2.06 [cm]
Pipe Grade	70 Grade
Specified Min. Yield Strength, SMYS	4942.57929[kg/cm ²]
Entry Angle	10 [deg]
Exit Angle	6 [deg]
Length of Crossing, L	175584 [cm]
Poisson's Ratio, μ	0.3
Radius of Curvature, <i>R</i>	100,000 [cm]
Young Modulus, E	20,400,000 [kg/cm ²]
Radius from Neutral Axis, y	45.72 [cm]
Pressure at Final Hydro-Test, P	127.5 [kg/cm ²]



Figure 2. Section outline.

serve as the key-points. The pipeline was assumed to move along the x-y direction and therefore no co-ordinate was given for the z-direction. The radius of curvature for the sag-bends was modeled as fillet between lines 1, 2 and lines 2, 3 respectively. For this model, the radius of curvature for both sag-bends was expected to be same, with reference to the design profile. However, the exit and entry angles were different.

The number of pipe section mesh was defined for the model. The "SECNUM" command was used to identify the particular section for meshing. This command reads the section division as the amount of mesh sizes. **Figure 3** shows the pipe section divisions used for the pipe meshing.

The "LMESH" command was used to generate nodes and line elements along lines. For this model, meshing was done on all the lines by selecting "ALL".

2.3. Solutions

2.3.1. Loads and Boundary Conditions

The "ANTYPE" command was used to specify the type of analysis and the restart status. For this model, the static analysis was required. The "DK" command was utilized to state the degree of freedom at the key points. For this model, at all the key points, only one degree of freedom was permitted which was in the x-direction. The y- and z-directions were constrained. The "FK" command was used to define the applied load in the x-direction at the key points. This applied load is the force of the rig which pulls the pipe in the x-direction. The "ALLSEL" command was used to select all entities.



Figure 3. Pipe size meshing.

2.3.2. Non-Linear Solution

The "NLGEOM" command was used to set out the large displacement static analysis option for the model. The time control for maximum (100), minimum (10) and model's number (100) of sub-steps for the basic solution control were also set.

Under the solution options, the "program chosen solver" and number (N) of subsets—1000 were selected for the equation solvers. Under the non-linear solution options of the solution controls, the highest number of iterations (1000), line search and DOF solution predictor was logged in and selected. The solution was generated using the macro file. A screen shot of the non-linear solution while converging can be viewed on **Figure 4**.

2.4. Theoretical Equivalent Resultant Stress

The equivalent resultant stress or Von Mises stress, expressed as a combination of all the stresses acting on the walls of the pipe includes: bending, longitudinal, hoop and radial stresses. However, the radial stress for thin wall material can be taken as negligible. Hence, the equivalent resultant stress can be expressed as Equation (1).

$$S_{theory} = \sqrt[2]{(\sigma_l^2 - (\sigma_l * \sigma_h) + \sigma_h^2)}$$
(1)

(2)

where bending stress, hoop stress and longitudinal stress are defined by equations (2), (3) and (4), respectively.

 $\sigma_b = E * \frac{y}{R}$

1) Bending stress,



Figure 4. Non-linear solution of the macro file in-progress.

2) Hoop stress,

$$\sigma_h = \frac{P * OD}{2 * t} \tag{3}$$

3) Longitudinal stress,

$$\sigma_l = (\mu^* \sigma_h) + \sigma_b \tag{4}$$

After running the ANSYS model using the relevant input parameters, the maximum equivalent stress obtained was S_{ANSYS} . The percentage error when comparing the resultant equivalent stress values from both theoretical calculation and ANSYS model was evaluated using Equation (5).

Percentage Difference =
$$\left(\frac{S_{ANSYS} - S_{theory}}{S_{ANSYS} + S_{theory}}\right) * 100\%$$
 (5)

For the ANSYS model validation and investigate the effect of the HDD radius of curvature on the pipeline stresses, the internal pressure was kept constant at P= 127.5 kg/cm² and different *R*-values (90,000 cm $\leq R \leq$ 120,000 cm) were applied to obtain different S_{theory}, S_{ANSYS} and percentage (%) difference which results were shown in **Figure 5**. Similarly, to investigate the effect of internal pressure on the HDD pipeline stresses, the pipe radius of curvature was kept constant at R = 100,000 cm while the internal pressure applied was varied in the range 90 kg/cm² $\leq P \leq$ 127.5 kg/cm² to obtain different S_{theory}, S_{ANSYS} and percentage (%) difference and the results shown in **Figure 6**.

3. Results and Discussion

3.1. Stress Validation—Effect of Radius of Curvature

Figure 5 showed the radius of curvature and the corresponding maximum equivalent stress obtained from theoretical method and ANSYS, respectively. From the **Figure 5** it can be observed that the percentage difference between the theoretically calculated maximum resultant stress and the maximum stress obtained from the ANSYS model was less than 10% with a maximum difference of 3.49% at a radius of curvature of 90,000 cm (900 m) and a minimum % difference of 0.22% at a radius of curvature of 110,000 cm (1100 m). Furthermore, the percentage error followed a parabolic distribution with respect to the radius of curvature, with a minima at R = 110,000 cm as shown in **Figure 5**.

3.2. Stress Validation—Effect of Internal Fluid Pressure

Figure 6 showed the variation of Von-Mises (maximum equivalent stress) with respect to internal fluid pressure of the pipeline. The results in **Figure 6** showed a comparison between the theoretically calculated and ANSYS equivalent resultant stress. From **Figure 6**, it can be observed that the percentage difference between the theory and ANSYS results was less than 10% with a maximum difference of 9.81% at an internal pressure of 90 kg/cm² and a minimum difference of 1.44% at an internal pressure of 127.5 kg/cm².



Figure 5. Effect of radius of curvature on equivalent resultant stress at $P = 127.5 \text{ kg/cm}^2$.





3.3. Cross Sectional Area Validation

A comparison was also done between the theoretically calculated cross-section area of steel pipe and the ANSYS model cross-sectional area. The theoretically calculated cross-sectional area of the pipe is given as Equation (6).

$$A_s = A_e - A_i \tag{6}$$

where A_e and A_i are the external and internal cross-sectional area of the pipe, respectively; and A_s was calculated as $A_s = 579.12 \text{ cm}^2$.

From **Figure 3**, the cross-sectional area of the ANSYS model cross sectional area was obtained as 578.439 cm². Therefore, the percentage difference in the cross-sectional area was obtained as:

Percentage Difference =
$$\left(\frac{579.12 - 578.439}{579.12 + 578.439}\right) * 100\% = 0.059\%$$

3.4. Element Numbering

The pipeline was discretized into 24 elements with 3 node points each by the ANSYS program for FEA to be done on each element. However, the element numbering was not arranged sequentially by APDL, but as shown in **Figure 7**.

3.5. Stress Contour Diagram and Maximum Stress Distribution

The stress contour diagram for the equivalent stress or Von Mises stress was plotted to investigate the stress distribution on the pipe. **Figure 8** showed a plot of the stress contour diagram for the model. The diagram showed that the maximum and minimum equivalent stress experienced by the pipeline was displayed on the top side and bottom side of the pipe, respectively.

Figure 9 shows a plot of the maximum stress induced on each element. Four sides of a single element were represented on the graph, namely: the top, bottom, left and right sides of the pipe. This was to identify the element with the highest maximum stress and also know the stress distribution along the sides of the pipe.

It can be observed from Figure 8 and Figure 9 that the maximum stress of 2550.08 kg/cm^2 was induced at the top-side on element number 21. This element was on the curved section close to the entry point where the angle of 10° was



Figure 7. Element numbering on ANYSYS Mechanical APDL.



Figure 8. Stress contour diagram.



Figure 9. Maximum stress per element around the pipe

used and which was much higher than the 6° used on the exit point. Although the radius of curvature was the same on both sides (entry and exit side), the maximum stress was experienced by the side with the highest angle. The bottom-side maximum stress 2399.9 kg/cm² was obtained at element number 2. Also, it can be observed from the stress contour diagram, **Figure 8** and **Figure 9**, that the stress on the top and bottom sides of the pipe was considerably higher on the elements in the curved sections of the pipeline.

However, the stress at the left-side and right-side of the pipeline was relatively

uniform at approximately 888 kg/cm² all through the pipeline regardless of the section where it was located. It would have been easily concluded that the stress at the curved sections would be higher than all other sections all-round the pipe, it can be seen that this was not the case.

3.6. Minimum Stress Distribution

Figure 10 showed the minimum stress distribution around the pipe for each element which also indicates that the stress distribution along the sides of the pipe all through the pipeline was relatively uniform regardless of the location. However the top and bottom minimum stress varies considerably at the curved sections. The minimum stress at the curved section indicates a lower stress level on the elements with maximum stresses.

The topside minimum stress was 17.237 kg/cm^2 at element number 21, while the bottom-side minimum stress was 96.784 kg/cm^2 obtained at element number 2. The left-side and right-side minimum stress remained uniform at approximately 888 kg/cm^2 irrespective of the element number.

3.7. Deflection Plot

A nodal solution of the pipe deflection was plotted to show the deflection at each section of the pipe. From **Figure 11** it can be observed that the maximum and minimum deflection experienced by the pipe was 413.942 cm and 0.000486 cm, respectively which were displayed on the top left-hand side of the **Figure 11**. The exact point of maximum deflection can also be observed in **Figure 11**. The location point of the maximum and minimum deflection point for the pipeline system was shown in **Figure 12**.



The plot of the pipeline deflection was shown on Figure 11. This plot in Figure 11

Figure 10. Minimum stress per element around the pipe.



Figure 11. Maximum deflection plot from the model.



Figure 12. Maximum and minimum deflection point.

indicates that the maximum deflection on the pipeline was experienced at the point of maximum stress level. While, **Figure 12** showing the points of the maximum and minimum deflection showed that the minimum deflection was obtained at the entry point of the HDD installation.

4. Conclusion

Hence, ANSYS can be used as veritable tool for the analyses and design of a pipeline system to be installed by horizontal directional drilling (HDD). The results obtained from this study using the ANSYS model were corroborated those of theoretical methods. The point of curvature with the higher angle (that is, if both points of curvature are the same and the entry and exit angles are different) experiences the maximum induced stress. Maximum deflection of the pipeline should be expected at the point with maximum stress as this was due to the high bending moment experienced at this point. On a particular pipeline system, the maximum and minimum stresses were induced at the pipe top-side and bottom-side, respectively; while the stresses at the sides of the pipeline remained relatively uniform with slight variation.

Conflicts of Interest

The authors declare no conflicts of interest regarding the publication of this paper.

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