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Modeling of the Changes in Flow Velocity on Seawalls under Different Conditions Using FLOW-3D Software

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Abstract

Seawalls are among protective structures that are constructed for decreasing the level of wave force and/or protecting from other structures. In this regard, more accurate investigation of these structures takes great importance from different perspectives. This research investigates the change of the velocity on seawall crown by considering the obstacles in different layouts and slopes. FLOW-3D has been used in this research for modeling. The results of the modeling show that the existence of obstacles has a determinative role in decreasing flow rate in the crown of seawalls. Also, as it was expected, the slope factor on upstream seawalls is very determinative in decreasing this rate such that the lowest velocity on the wall occurs in D-state layout and the slope of 45°.

Keywords

Flow Velocity, Seawall Crown, Modeling, FLOW-3D

1. Introduction

Seawalls are among the protective structures that are constructed for decreasing the rate of wave force and/or protecting other structures. Lighthouses are among the structures that are usually protected by seawalls. There-

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fore, in addition to the importance of the volume of passing-water upon seawalls, the velocity rate of passing-flow on crown for this structure is very important because of the importance of velocity factor in creating momentum and impulse on the structure behind these walls. Basically, the creation of obstacle in upstream slope and the rate of upstream slope can be very effective in decreasing the amount of this velocity. However, this issue must be accurately investigated by modeling to reach an optimum layout of obstacles in a specific slope. In this research, three-dimensional model of FLOW-3D is used for investigating the mentioned issue [1].

2. Research History

Several studies have focused on waves overtopping marine structures. These methods have consistently correctly predicted overtopping of structures to protect the coast against sea waves. By 2002, nearly 6500 tests had been conducted; physical models of regular waves have also been carried out in the USA [2]. The most complete set of random waves was completed by Owen (1980). Owen carried out a number of physical model tests to study overtopping and the relationship between the height of the sea wall and degree of overtopping [3]. He showed that the degree of overtopping depends on environmental conditions such as wave height and wave period, as well as on the geometry and type of structural material. The combination of these factors should be investigated. Von Meyer and Duval (1992) carried out another series of studies [4].

3. Materials and Methods

In this research, 68 different geometries are given to the software for the modeling and they are briefly introduced in the following **Table 1**. These 68 different geometries include 4 different slopes, 4 different layouts and 4 different heights of obstacles and also 4 states without obstacle and only under different slopes [5]. Then, these different geometries and states are evaluated and analyzed under the same conditions using FLOW-3D three-dimensional model.

Table 1. Designation of variables.

Form	Roughness		Form of roughness			
		1	2	3	4	
	Without roughness		A1			
A	10 cm	A2-1	A2-2	A2-3	A2-4	
	20 cm	A3-1	A3-2	A3-3	A3-4	
	30 cm	A4-1	A4-2	A4-3	A4-4	
	50 cm	A5-1	A5-2	A5-3	A5-4	
В	Without roughness		B1			
	10 cm	B2-1	B2-2	B2-3	B2-4	
	20 cm	B3-1	B3-2	В3-3	B3-4	
	30 cm	B4-1	B4-2	B4-3	B4-4	
	50 cm	B5-1	B5-2	B5-3	B5-4	
С	Without roughness		C1			
	10 cm	C2-1	C2-2	C2-3	C2-4	
	20 cm	C3-1	C3-2	C3-3	C3-4	
	30 cm	C4-1	C4-2	C4-3	C4-4	
	50 cm	C5-1	C5-2	C5-3	C5-4	
D	Without roughness		D1			
	10 cm	D2-1	D2-2	D2-3	D2-4	
	20 cm	D3-1	D3-2	D3-3	D3-4	
	30 cm	D4-1	D4-2	D4-3	D4-4	
	50 cm	D5-1	D5-2	D5-3	D5-4	

4. Numerical Models

The FLOW-3D software is a powerful hydraulic simulator application in the field of fluid dynamics with three-dimensional flow field analysis. The equations governing in the model, like other similar models, are Navier-Stokes equations and the conservation of mass equation [6].

In order to model the channel in this application, we need to define the General Conditions (including the simulation of all systems), physical conditions, geometry and model-solving network, adjusting the outputs and its related options. Celsius Degrees was chosen for system units, SI, and temperature.

In physical terms, the software allows that, according to the principles of the physics governing the phenomenon, the relevant conditions are chosen. The physical conditions governing this study are gravity and viscosity and turbulence. The turbulence in this software is stimulated by five models and the model used in this research was *Re-Normalisation Group* (RNG). In this model of turbulence, constant values which were experimentally calculated in K-model are implicitly derived [7].

After that, the fluid should be defined. The selected fluid of this study is the 20-degree-Celsius water [8].

The next step is to define the geometry and resolve network, which is of importance in simulation [9]. The FLOW3D enables the user to depict a lot of fluid phenomena with the tools available in the software. Defining the channel geometry, resolve network should be defined. The defined resolve network of the software is in the form of regular (cubic) resolve network including network size, number of cells and their dimensions in three coordinates of X, Y, and Z and boundary conditions. The smaller the size of network cell dimensions and the greater their number, the higher the capability and precision of the program for simulation will be [10].

5. Results

As it can be observed in the different figures, the diagrams are in two types: first, direct output from software that includes the **Figures 1-4** and the other the **Figures 5-7** as the diagrams of the process of the changes. However, it must be mentioned that in the **Figures 1-4**, outputs from one of the slopes, as an example, are directly brought from software output.

With regard to the mentioned figures, it can be understood that the highest velocity rate is occurred in upstream slope of seawall while these velocities are maximum in upstream slope of the state without obstacle. The interesting point is that the lowest velocity usually exists on seawall crown.

As it can be seen from the **Figures 5-8**, in all the different slope states of upstream seawall, the highest flow velocity is related to the obstacle with 10 cm height and lowest velocity is related to the obstacle with 50 cm height. The reason is that the value of flow kinetic energy which is converted to potential energy because of the collision with obstacles increases by increasing the height of the obstacles. Therefore, when the height of

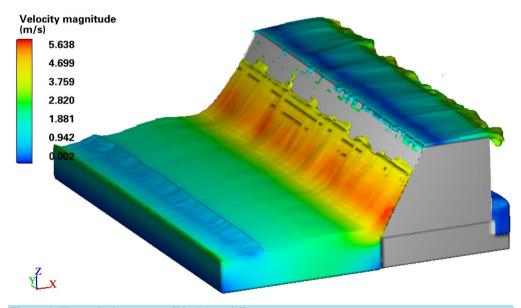


Figure 1. Flow velocity on seawall in A1 modeling.

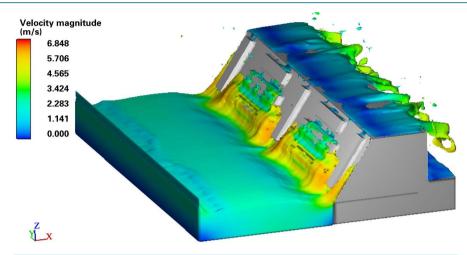


Figure 2. Flow velocity on seawall in A2-1 modeling.

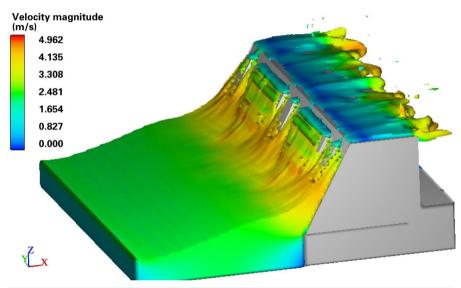


Figure 3. Flow velocity on seawall in A2-3 modeling.

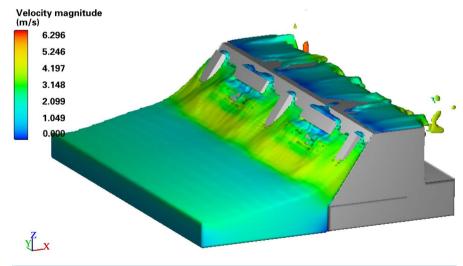


Figure 4. Flow velocity on seawall in A3-1 modeling.

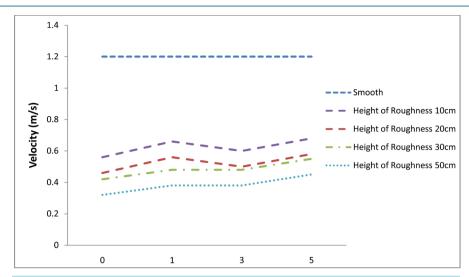


Figure 5. The changes of flow velocity in the seawall type A (61° slope).

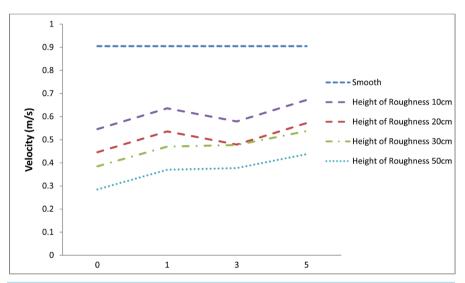


Figure 6. The changes of flow velocity in the seawall type B (56° slope).

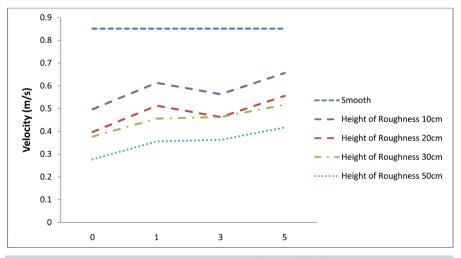


Figure 7. The changes of flow velocity in the seawall type C (51° slope).

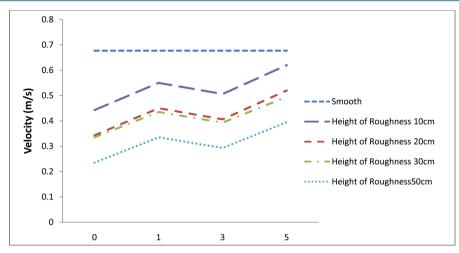


Figure 8. The changes of flow velocity in the seawall type D (45° slope).

corresponding flow kinetic energy in a moment is converted to the corresponding height of potential energy in the collision of flow to each obstacle, it causes flow velocity to be zero for a moment and then its velocity is increased after crossing the obstacle. Whatever the height of the obstacle is lower, there would be fewer momentary zero velocity states and the flow continues its moving with higher velocity.

6. Conclusion

Also, as it can be observed, the highest difference of velocity in all the figures is between the obstacles with 10 cm height and the obstacles with 50 cm height. Also, this amount of difference in velocity for difference between the obstacles with 10 cm and 20 cm heights is higher than that of the differences in the obstacles with 20 cm and 30 cm heights which can be related to the special conditions in flow hydraulic in that range of height.

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